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THE 99TH ANNUAL

# PURDUE ROAD SCHOOL

## **Toward Long Lasting, Durable Bridge Decks**

Acknowledge Contributions from  
Ron Walker, Neal Carboneau,  
Tony Zander, Tommy Nantung

Jason Weiss, [wjweiss@purdue.edu](mailto:wjweiss@purdue.edu), Purdue University  
Professor & Director of the Pankow Materials Laboratory



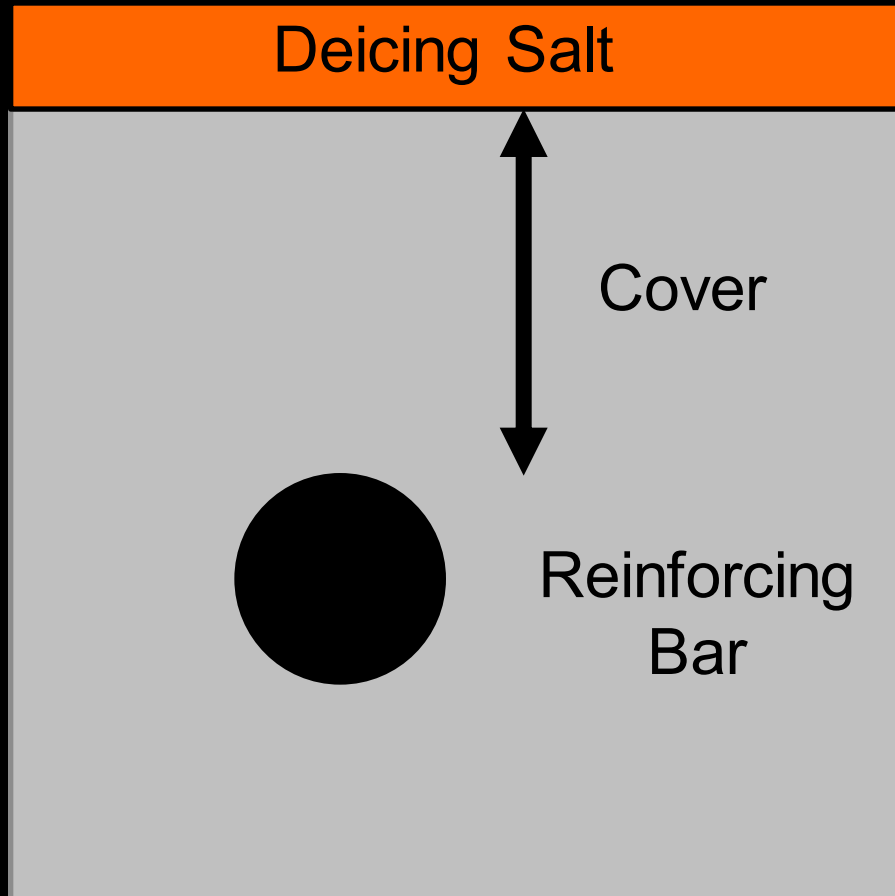
# Salt Usage Abounds

- Before 1940's, removal of snow and ice used plowing, salt and cinders
- 1950-60's bare pavement policy implemented
- 1970's deicing salts become widely used to maintain clear pavements
- US - salt is the most widely used deicing material with 8-12 million annual tons





# Durability Related to Transport

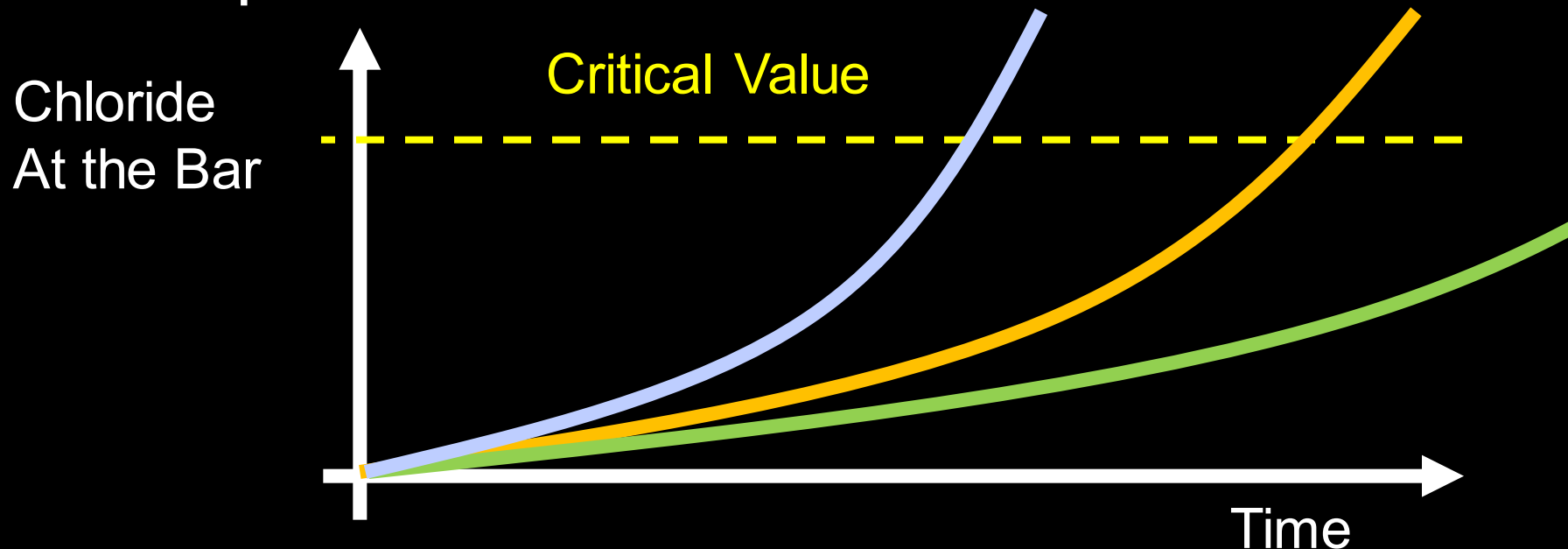


- Salt can be absorbed or can diffuse in the pores and when it reaches the rebar it can depassivate the steel and cause corrosion



# How Long Does for Corrosion to Start

- The chloride will migrate to the bar over time
- How long does it take to reach a critical level
- Depends on the quality of the concrete and the depth of the reinforcement







# Various Strategies to Extend Time

- Higher Strength
- Sealers
- Overlays
- Coated Bars

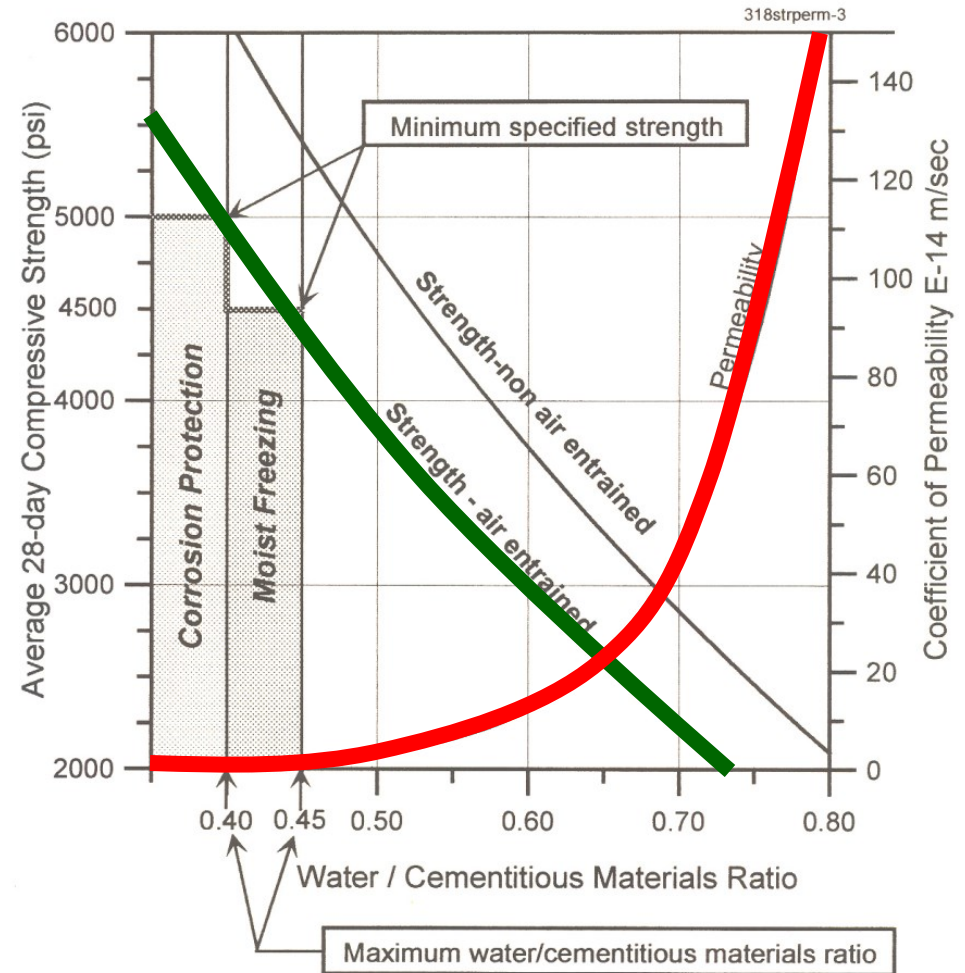




# Reducing Transport

- Strength is related to w/c; transport properties are even more strongly influenced
- WRA, SCM, SF all densify concrete
- Why is this the case and what else may change

After ACI 318 From Hover 2003

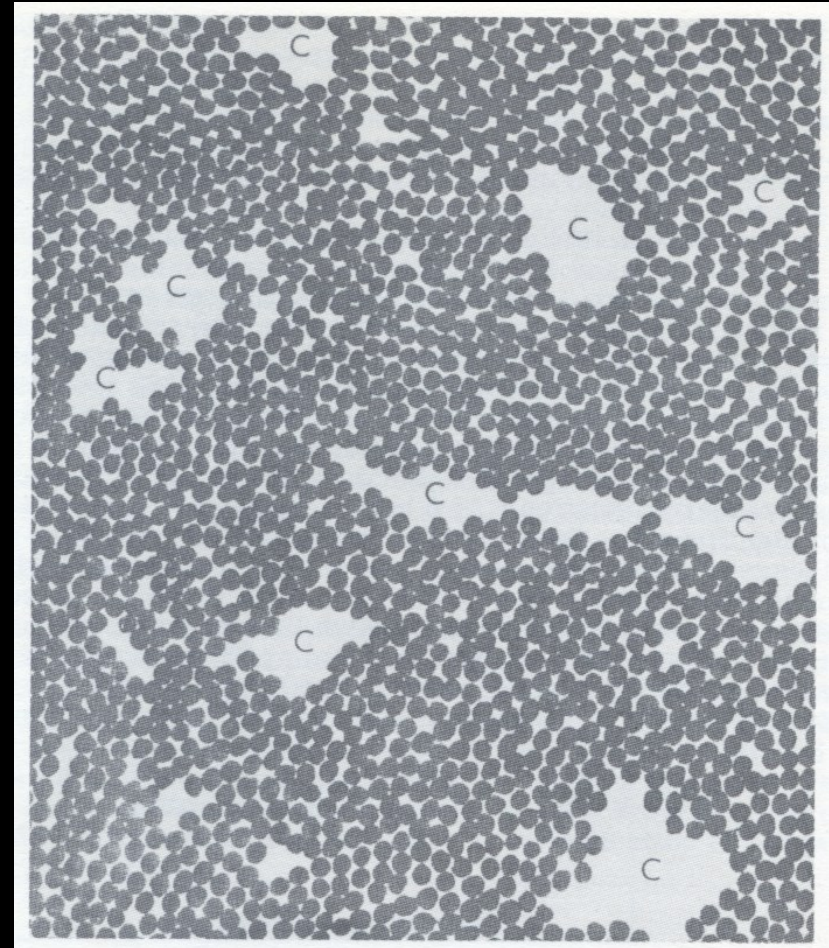
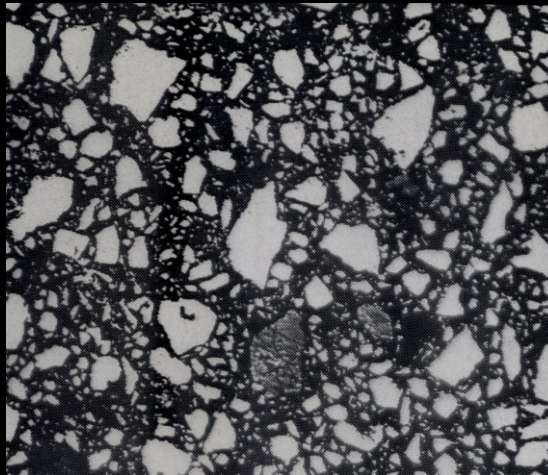






# A Pore Structure Argument for Why We Want Higher Strength Concrete

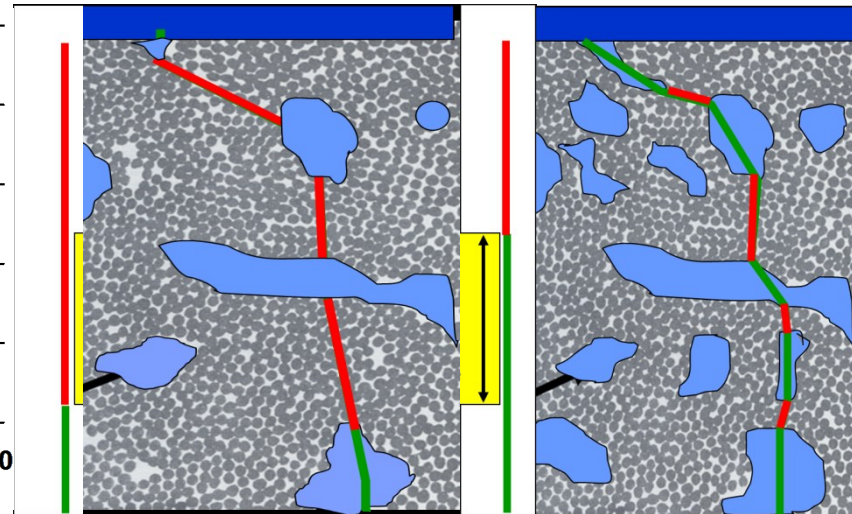
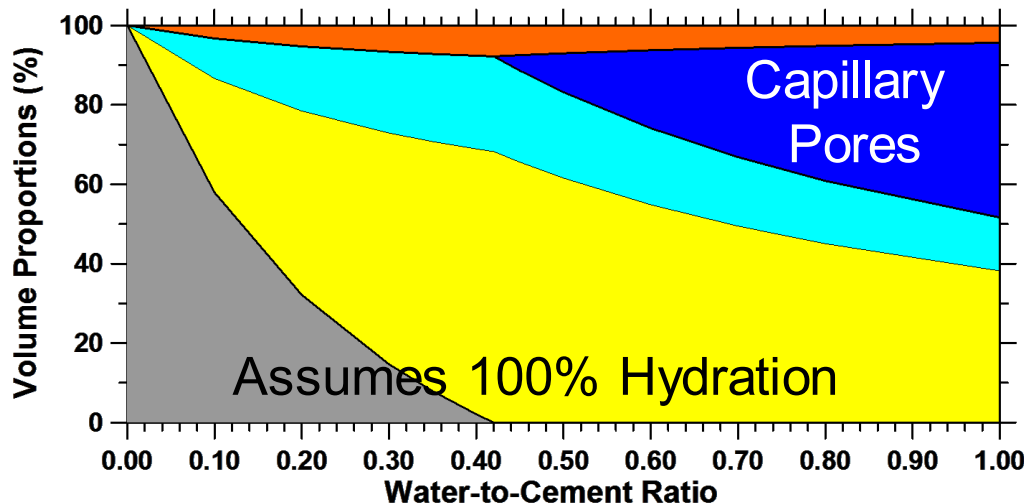
- Concrete is a Porous Material
- Two Types of Pores
  - Capillary Porosity (Big Pores Mix Water, Fill in Over Time)
  - Gel Porosity (Small Pores - Hydration)





# Transport in Large Pores, Shrinkage from Small Pores

- Transport occurs through capillary pores
- Capillary pores – large and connected
- Low w/c - smaller pores – improves strength and improves transport
- But what about the influence on other properties

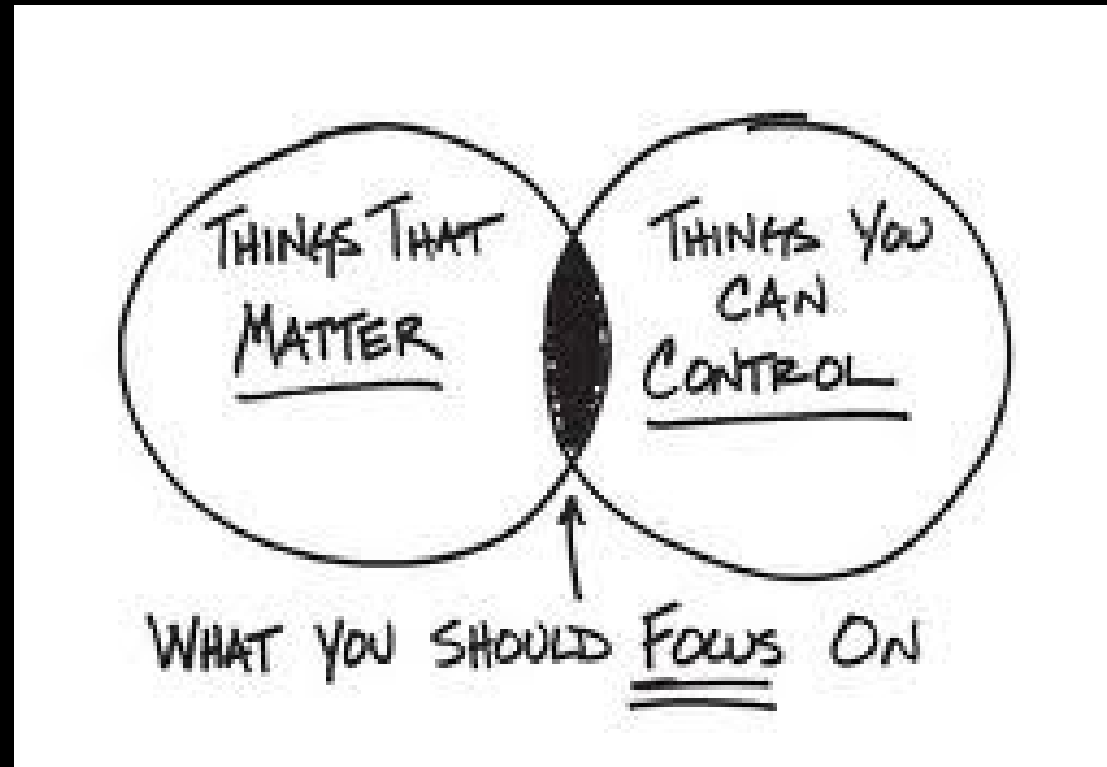






# What Can We Control

- We can control the capillary pores by controlling the w/c (SCM and WRA good)
- We should do this when we can
- Excess water leads to pores and increased transport
- Low w/c = HSC





# Higher Strength

## Why Do We Want It

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- “**High-strength** concrete is one of the most significant new materials available to federal, state, and local highway agencies.....  
With its improved **impermeability, durability, and accelerated strength** gain ..... an ideal material .....
- HSC **may be slightly more expensive** than normal concrete initially, but its greater strength means that HSC bridges may require **fewer supports, which could reduce overall costs.**



# Benefits of High Strength Concrete

## Advantages

- Higher Strength
- Rapid Strength Gain
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- Volume Stability
- Toughness
- Higher Modulus
- Lower Creep

## Disadvantages

- Costs



It'll knock your socks off..  
And it'll get'em whiter





# Asking for Higher Strength with the Best of Intentions....

**True or False:**  
Increasing  
Strength Improves  
Performance



.....Misconceptions of Using Lower  
W/C, Higher Strength Concrete

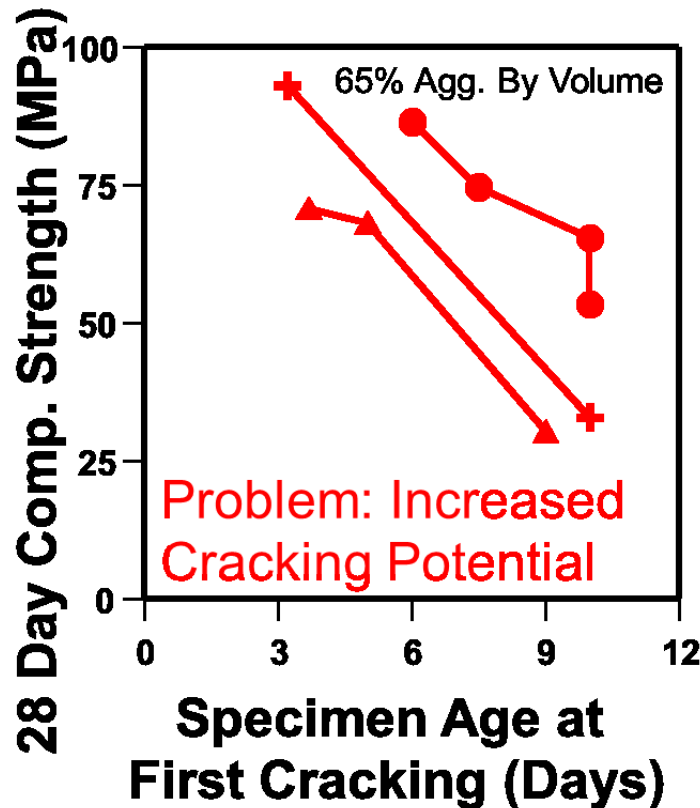


# Is All High Strength Concrete Evil ??

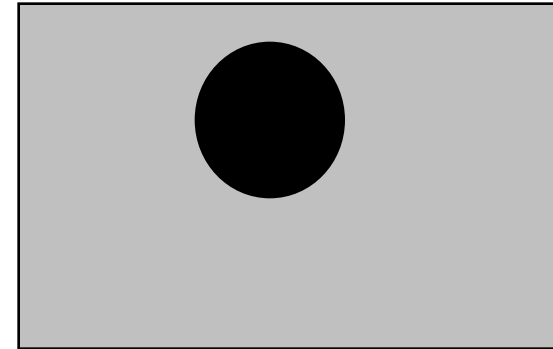


Obviously No  
High Quality Concretes  
Can Be  
Made with High  
Strength, However  
  
Be Prepared for What You  
Ask For  
Additional Problems  
May Arise

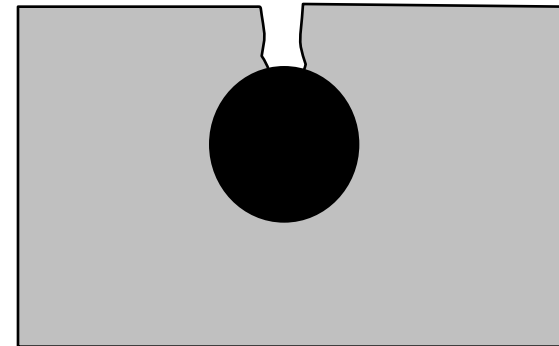
# Low Transport ..... Between the Cracks



Weiss 1997



High Strength/Low Permeability  
is Important If Uncracked



Not so Important once the  
Section has Cracks





## Advantages

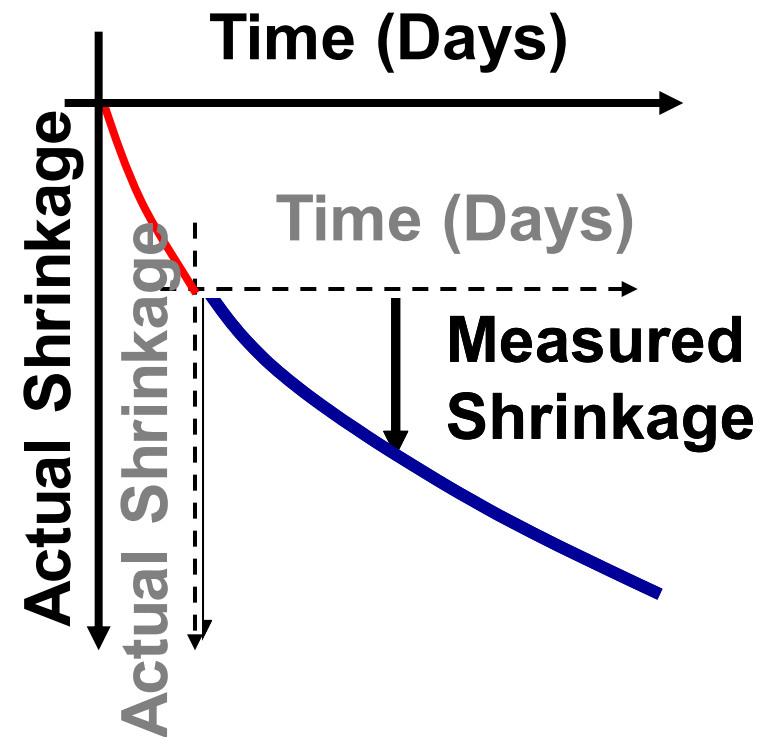
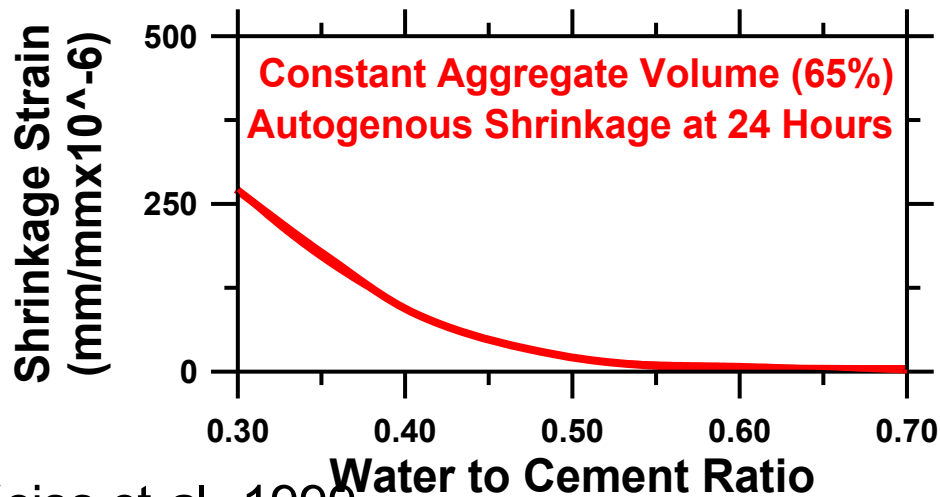
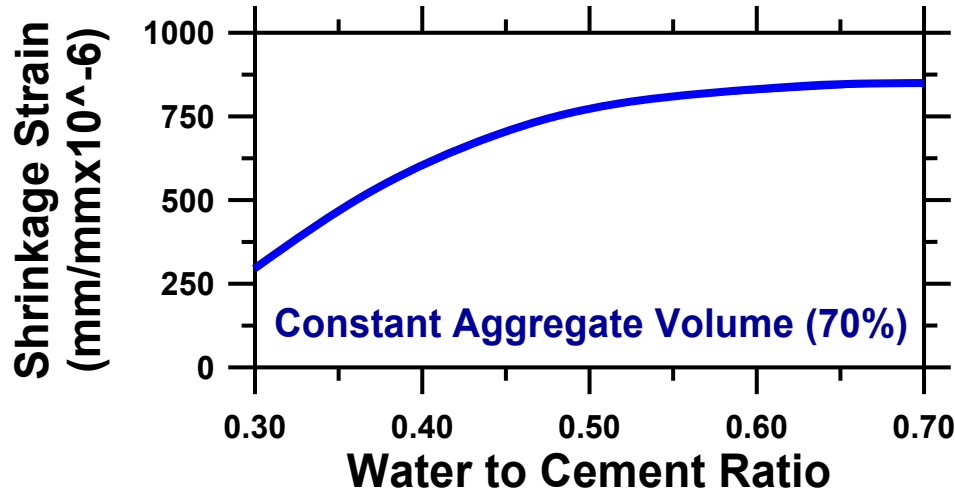
- Higher Strength
- Rapid Strength Gain
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- **Volume Stability**
- Toughness
- Higher Modulus
- Lower Creep

## Misconception #1

**Higher Strength  
Concrete Has  
Significantly Lower  
Shrinkage  
(Volume Stability)**



# Measuring Shrinkage Starting Time is Critical



Weiss et al. 1999

After Aitcin 1999



## Advantages

- Higher Strength
- Rapid Strength Gain
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- Volume Stability
- Toughness
- Higher Modulus
- Lower Creep

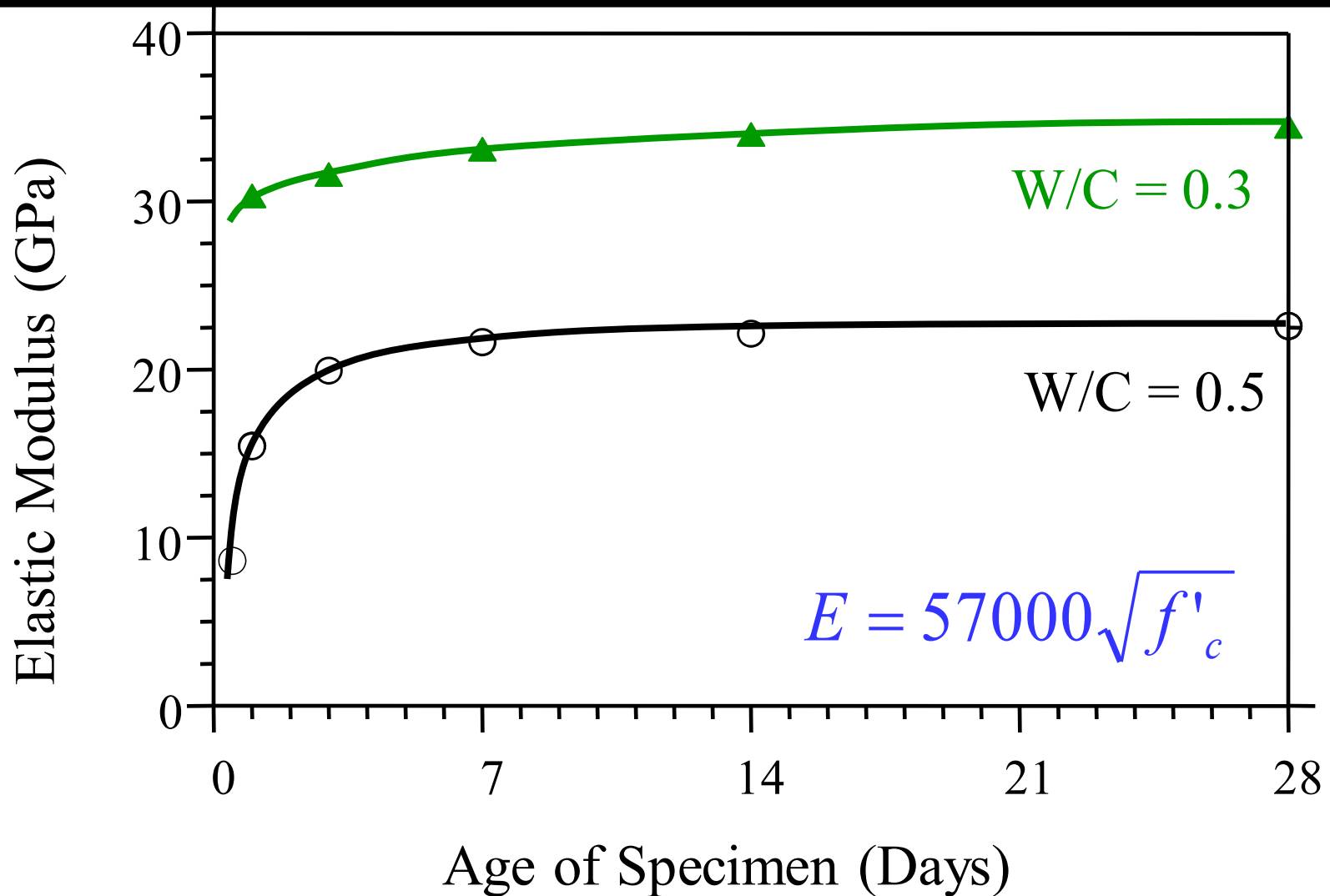
## Misconception #2

Higher Strength  
Has Higher  
Stiffness and  
That's Always  
Good





# Modulus Development



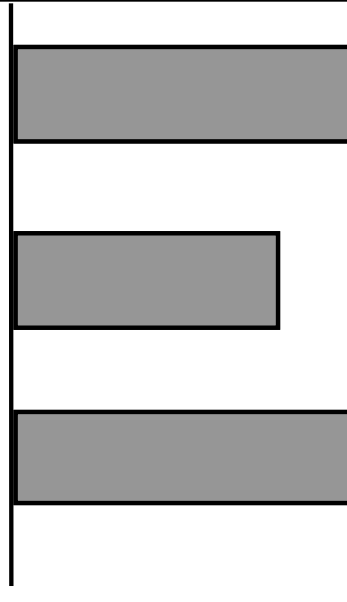


# Using Hooke's Law to Compute Stress

Initial Specimen

Shrinkage Effect

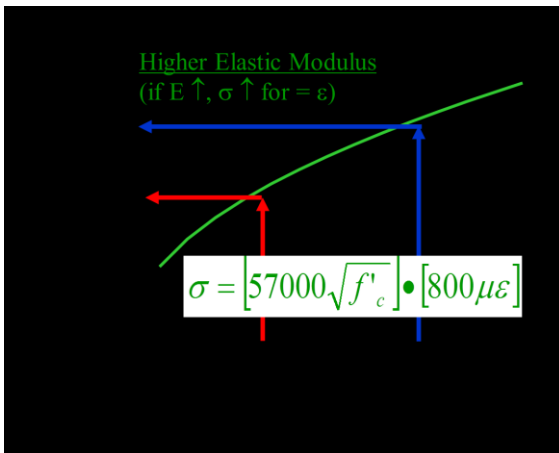
Restraint Effect



Initial Pavement

Disconnect it From  
The Subgrade

Apply Force to  
'Simulate' Subgrade



$$\sigma = E \epsilon$$

Assumed  
Shrinkage

$$\sigma = [57000\sqrt{f'_c}] \bullet [800\mu\epsilon]$$

Weiss et al. 1999



## Advantages

- Higher Strength
- Rapid Strength Gain
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- Volume Stability
- Toughness
- Higher Modulus
- Lower Creep

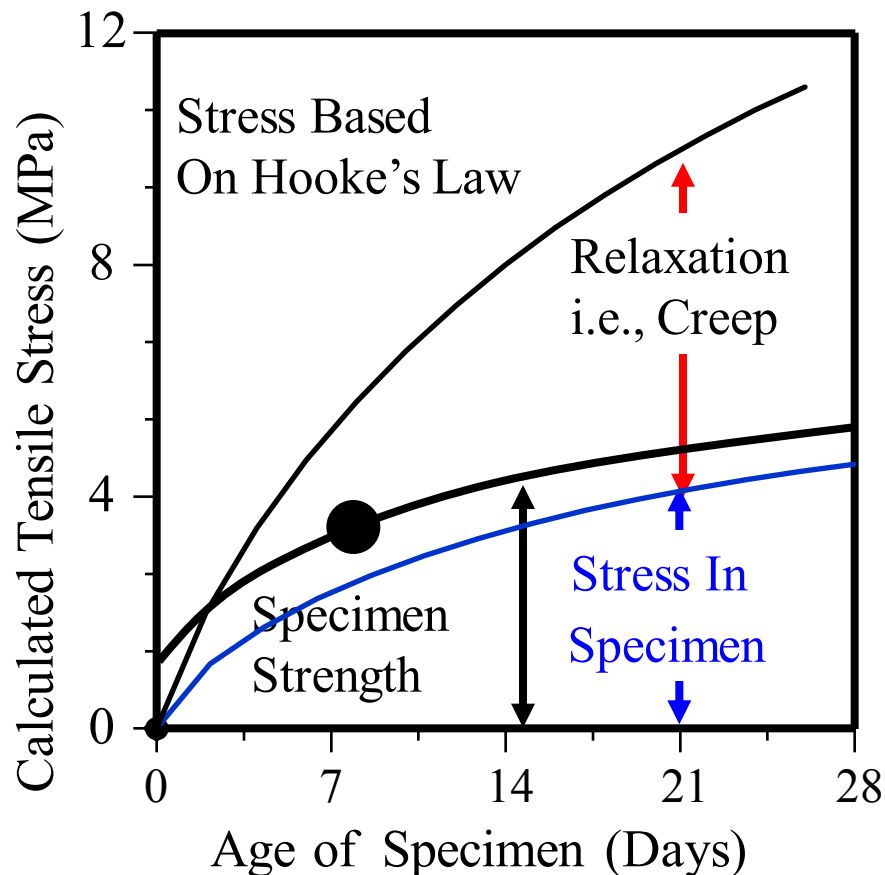
## Misconception #3

Higher Strength  
Has Lower Creep  
and That's Always  
Good

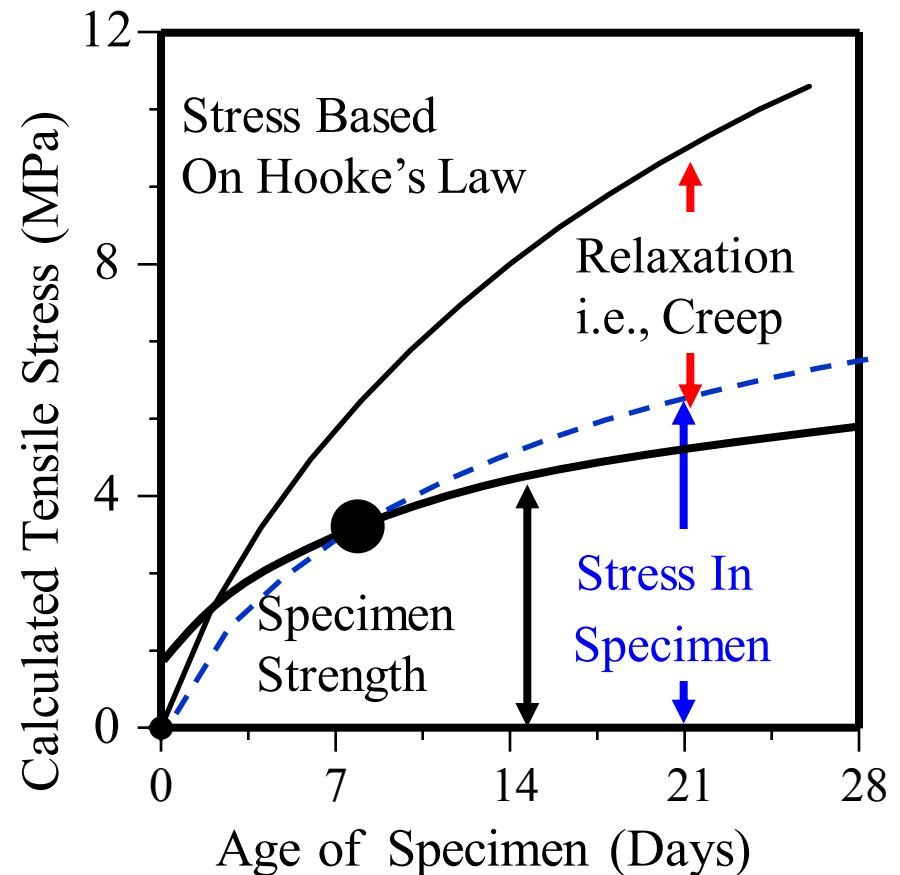




# Misconception: Lower Creep is Always Beneficial to Performance



Specimen with Lower Creep



Weiss et al. 1999



## Advantages

- Higher Strength
- Rapid Strength Gain
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- Volume Stability
- **Toughness**
- Higher Modulus
- Lower Creep

## Misconception #4

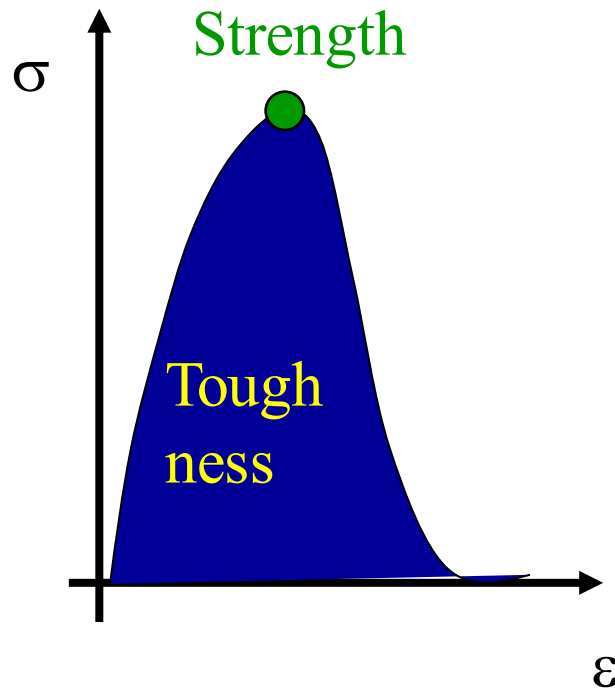
**Higher Strength is  
Tougher  
Than Conventional  
Concrete**



# Strength Versus Toughness



Arnold  
Strong

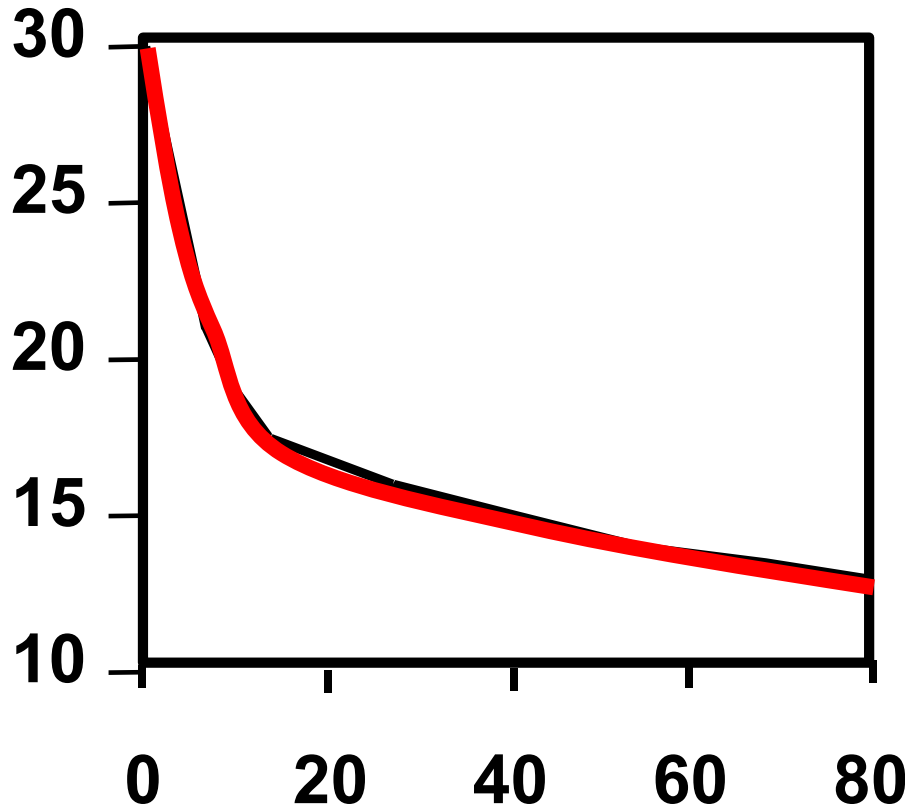


Clint  
Tough

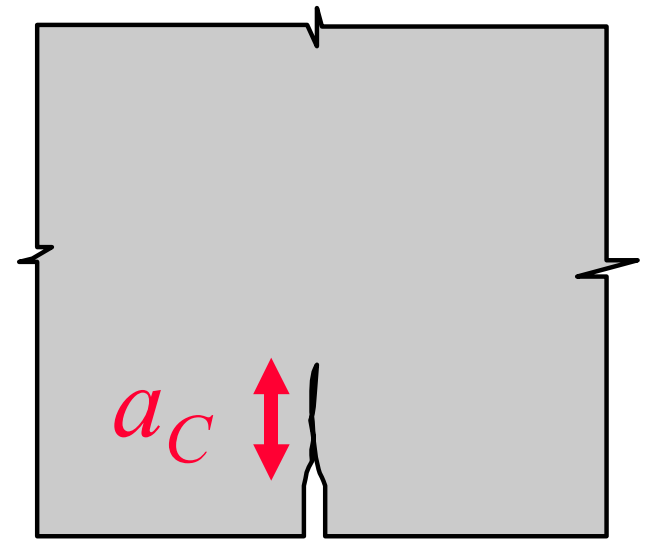


# Higher Strength Concrete is More Brittle

$$\text{Crack Ratio} = \frac{a_C - a_0}{D - a_0} (\%)$$



Comp. Strength (MPa)



Higher Strength is More Like Glass

Weiss et al. 1999



## Advantages

- Higher Strength
- **Rapid Strength Gain**
- Low Permeability
- Improved Durability
- Costs
- Less Members
- Ease of Placement
- Volume Stability
- Toughness
- Higher Modulus
- Lower Creep

## Misconception #5

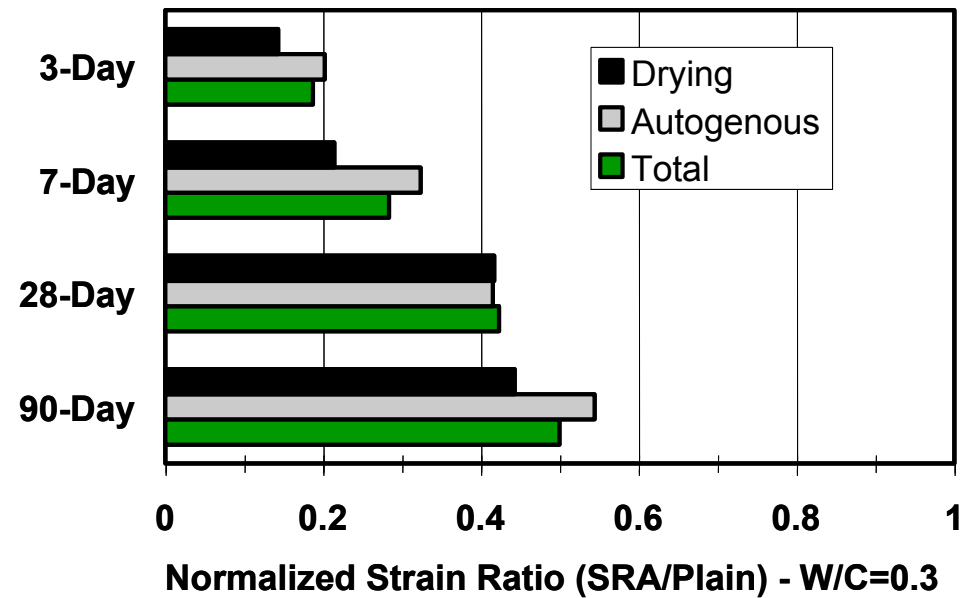
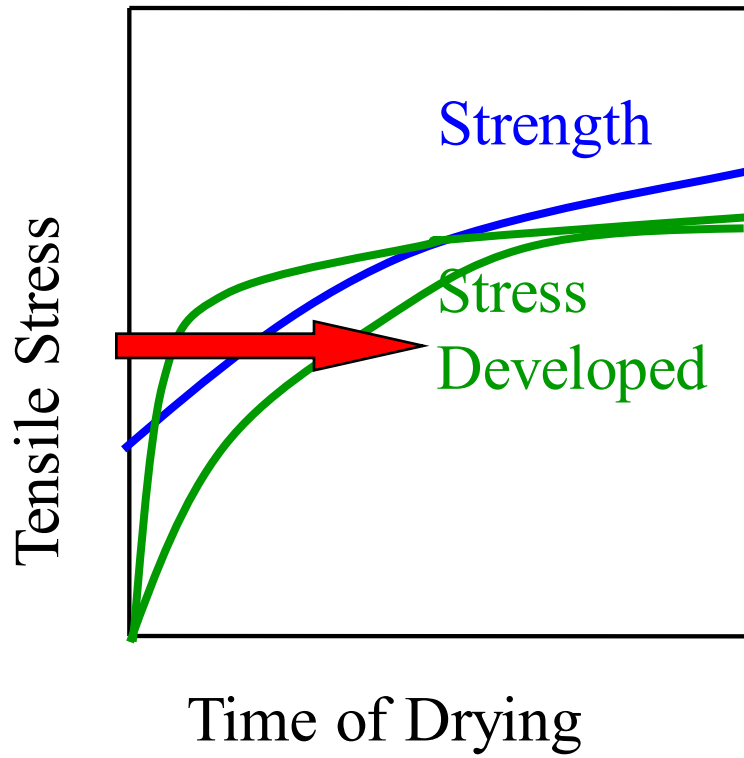
**Higher Early  
Strength, More Rapid  
Strength Gain is  
Always Better**





# Reducing Stress to Reduce the Potential for Shrinkage Cracking

Reduce Rate



Weiss et al. 1999



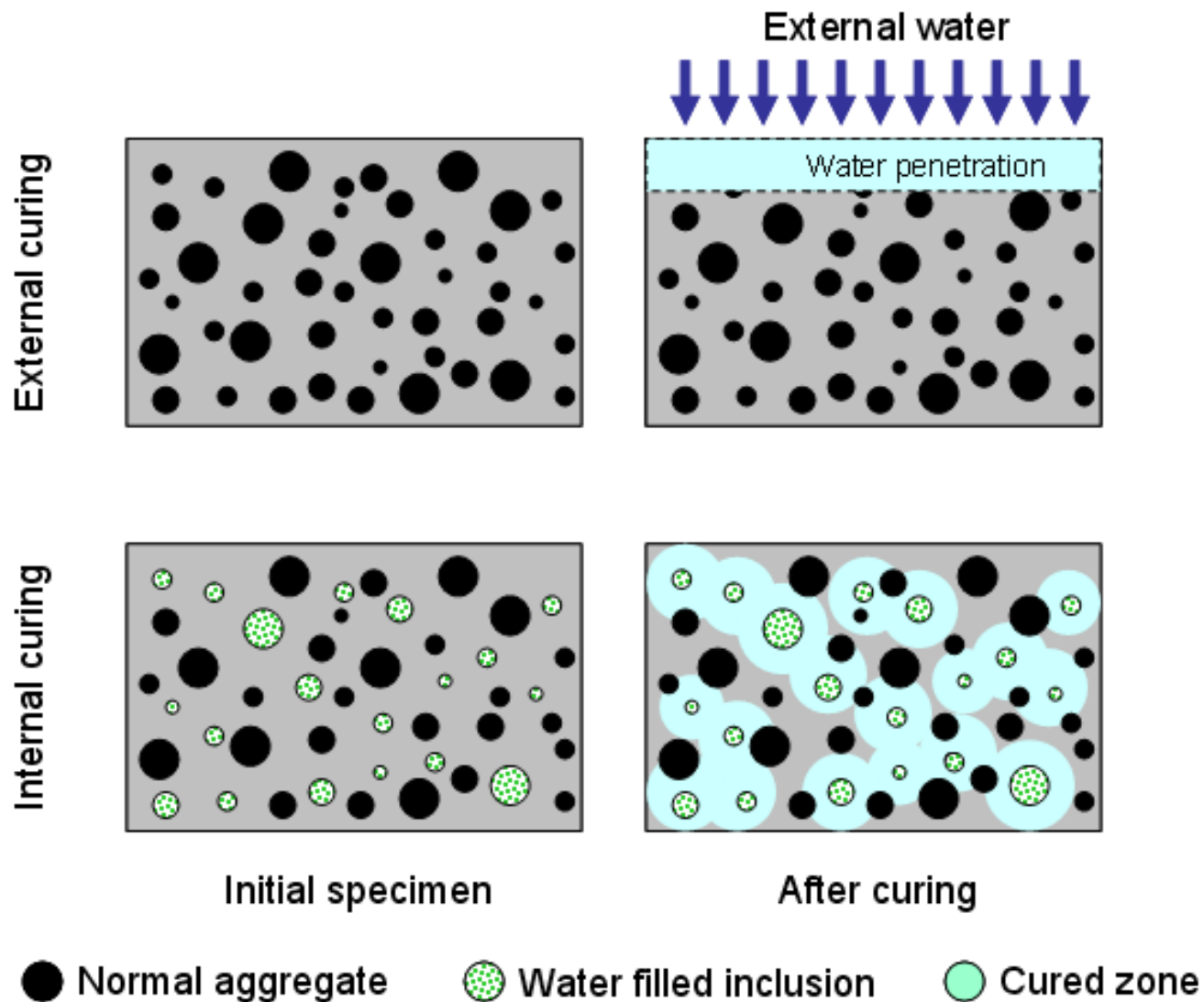
# Lets Collect our Thoughts

- People want HSC due to low transport
- HSC can lead to increased cracking
  - Increased shrinkage (typically not measured)
  - No substantial improvement in toughness
  - Reduced 'creep' (VE mtl) (reduced relaxation)
  - Increased stiffness
  - Rapid strength gain
- What do we want? low transport and low shrinkage and cracking
- How do we get it? – MP, SRA, Internal curing



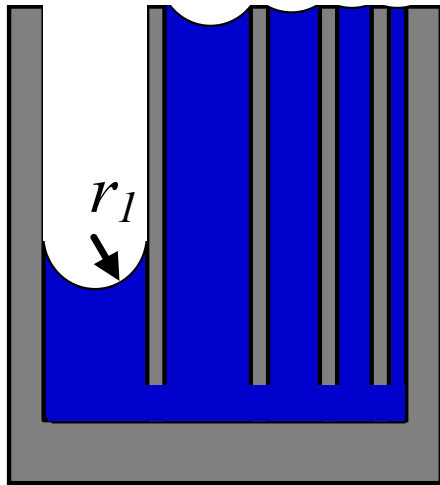


# Conventional and Internal Curing





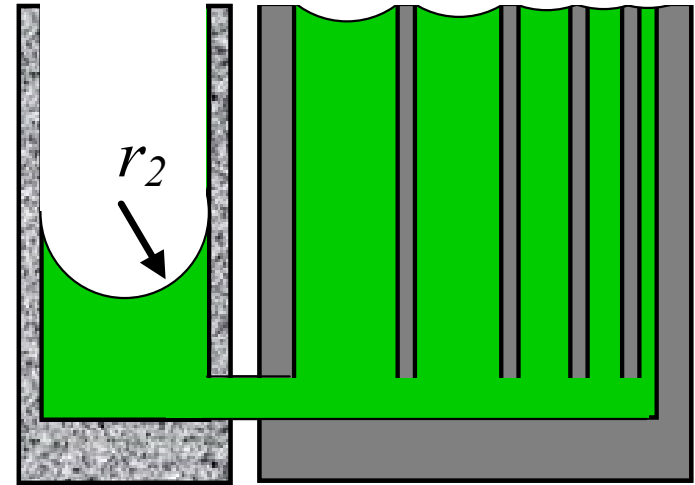
# Internal Curing and Pore Size



**Sealed - Plain**

A Similar  
Volume of  
Water is  
Depleted

LWA higher  
r in Agg



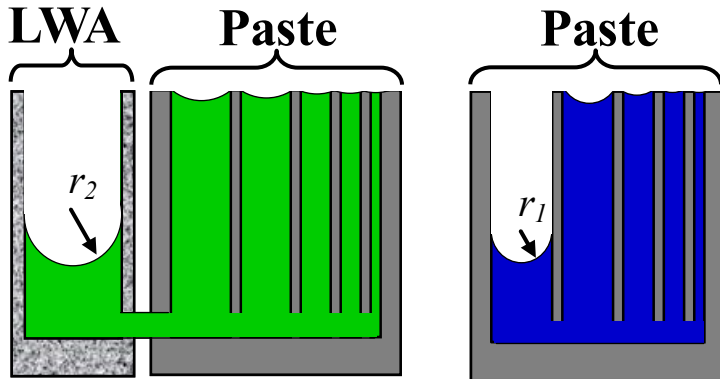
**Sealed - LWA**

- Prewetted LWA provides water to the paste and keeps a large pore full
- As a result the pore size that remains full is larger

$$\sigma = \frac{2\gamma \cos \theta}{r}$$



# Effect of Internal Curing on Shrinkage



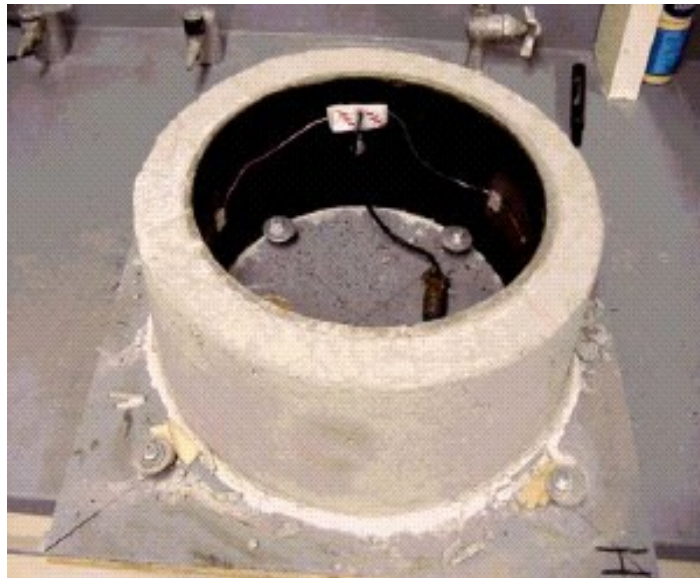
$$\epsilon_p = \frac{S}{3} \left( \frac{2\gamma}{r} \right) \left( \frac{1}{K_p} - \frac{1}{K_s} \right)$$

- Remaining water will fill in pores
- LWA Pores > Paste Pores
- Modified version of Mackenzie's equation
- Larger pores remain saturated - less shrinkage

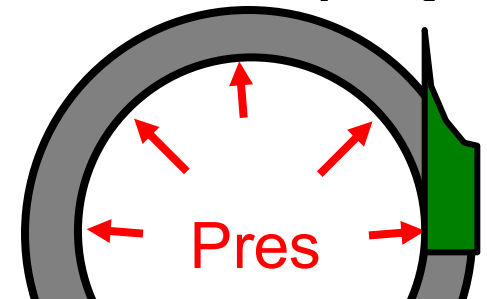
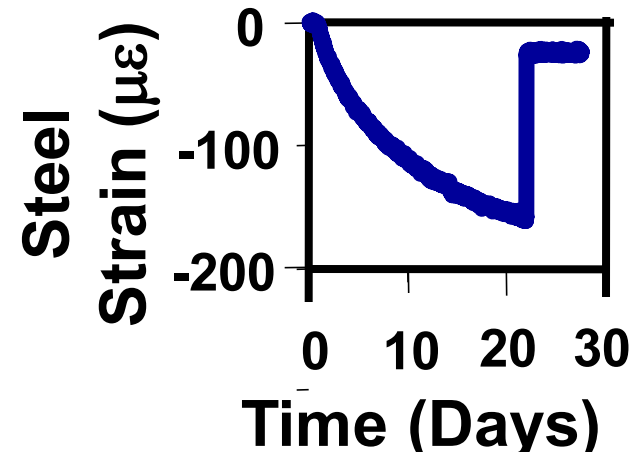
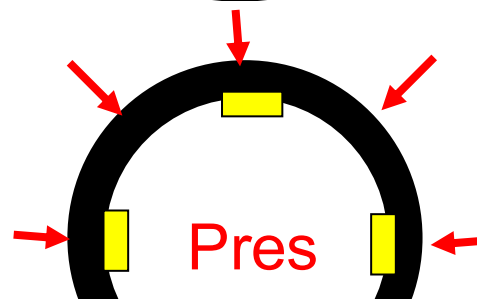
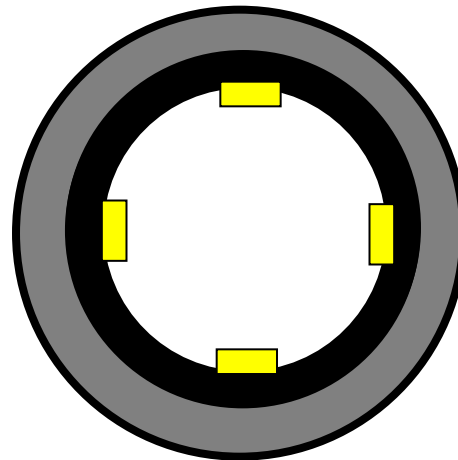




# Quantifying Stress with the Ring Test



Top & Bottom Drying  
or Perfectly Sealed

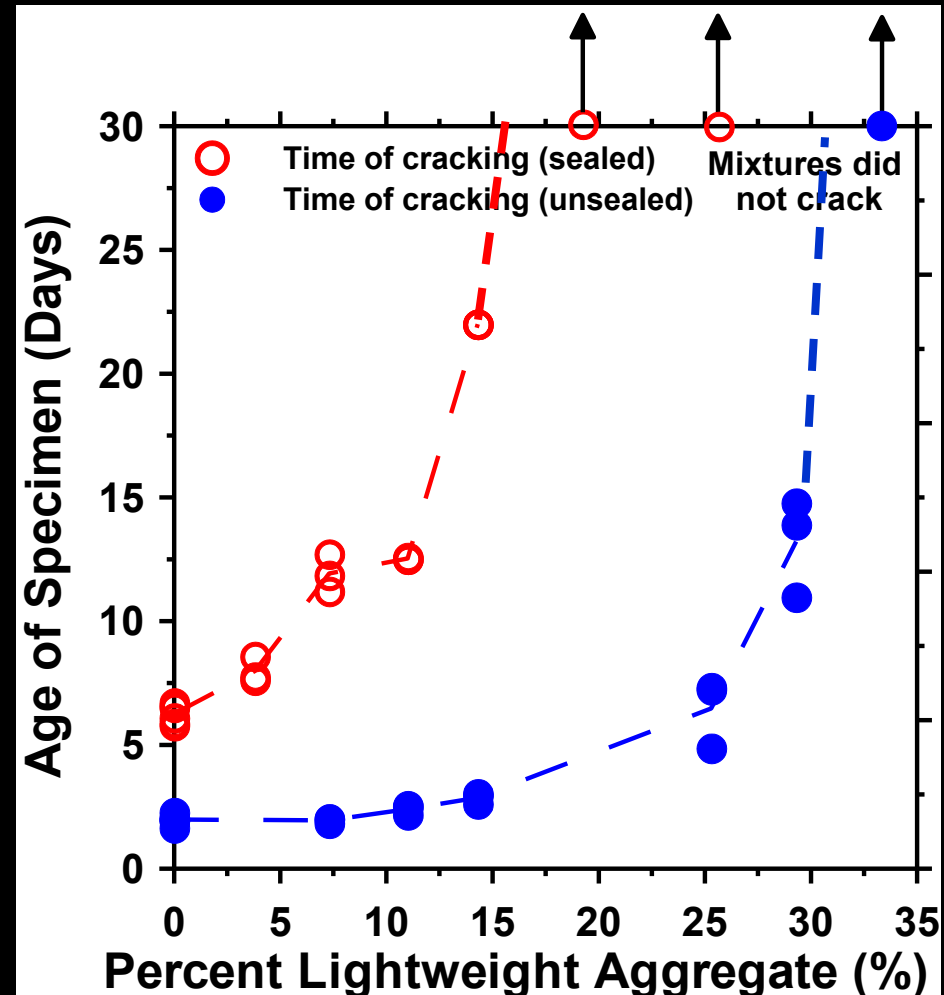


$$\sigma_{Concrete}(t) \Big|_{r=R_{IC}} = \epsilon_{Steel}(t) E_S \frac{(R_{OS}^2 - R_{IS}^2)}{2 R_{OS}^2} \frac{(R_{OC}^2 + R_{IC}^2)}{(R_{OC}^2 - R_{IC}^2)}$$



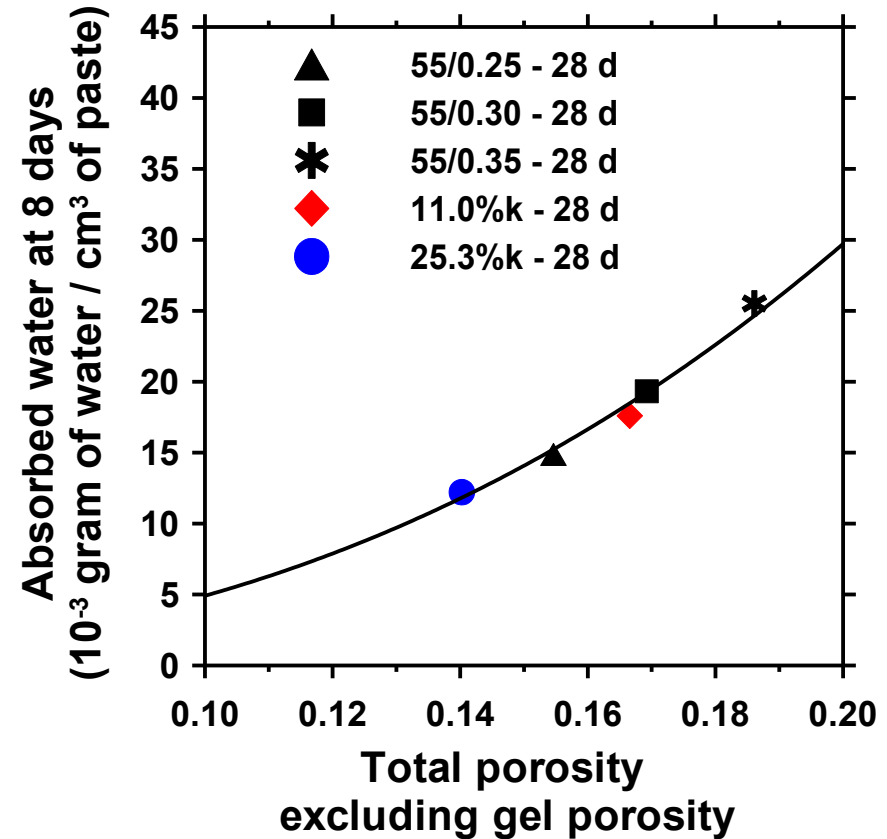
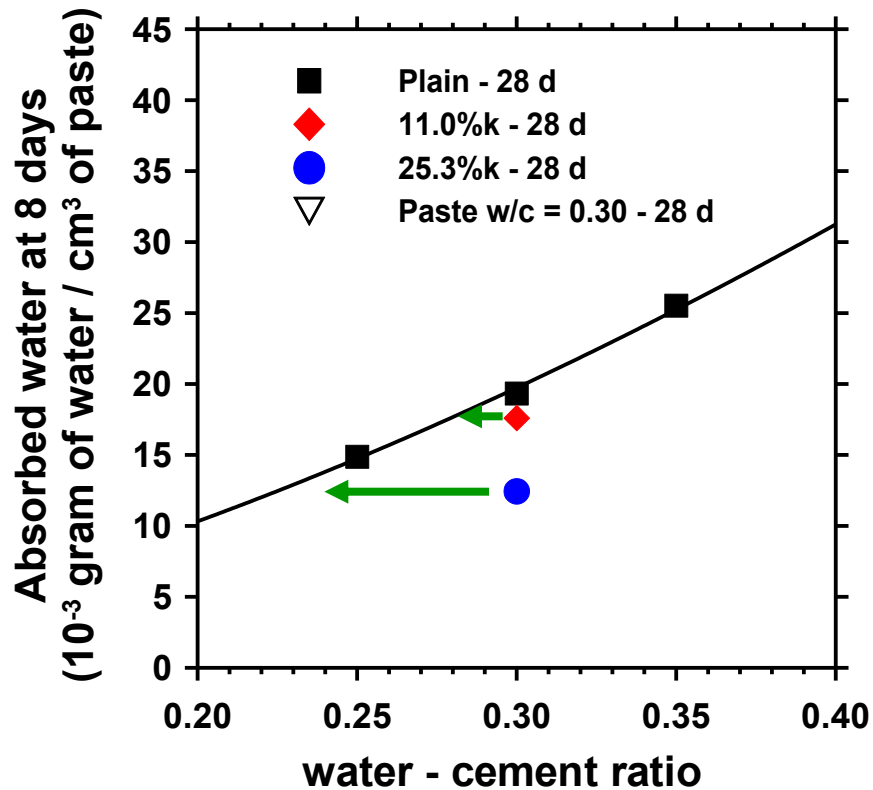
# Restrained Shrinkage

- We can see that cracking is more likely than in the sealed case
- Appears logical
- Shrinkage rate decreases and can improve crack resistance





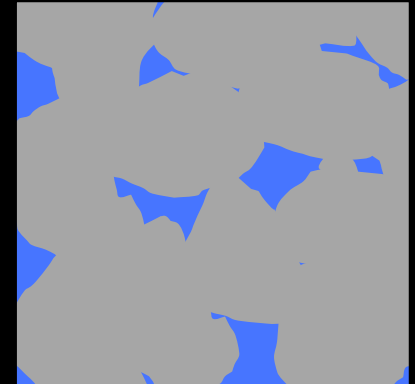
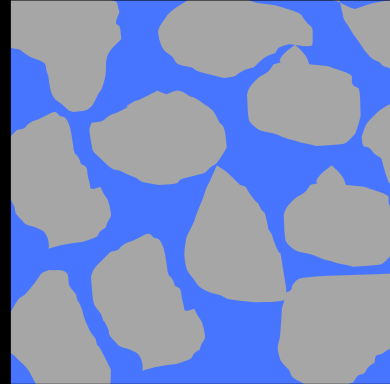
# Benefit of Internal Curing



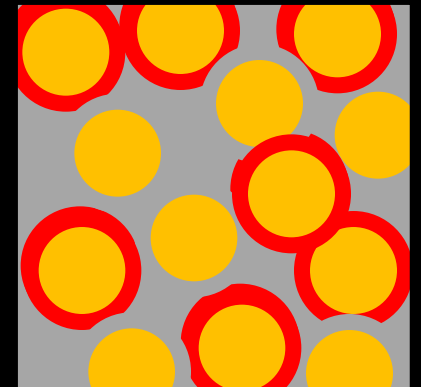
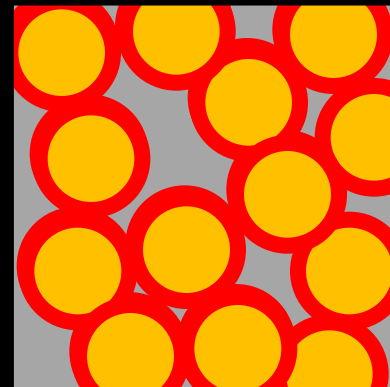


# Two Benefits

- Reduced porosity due to increased hydration

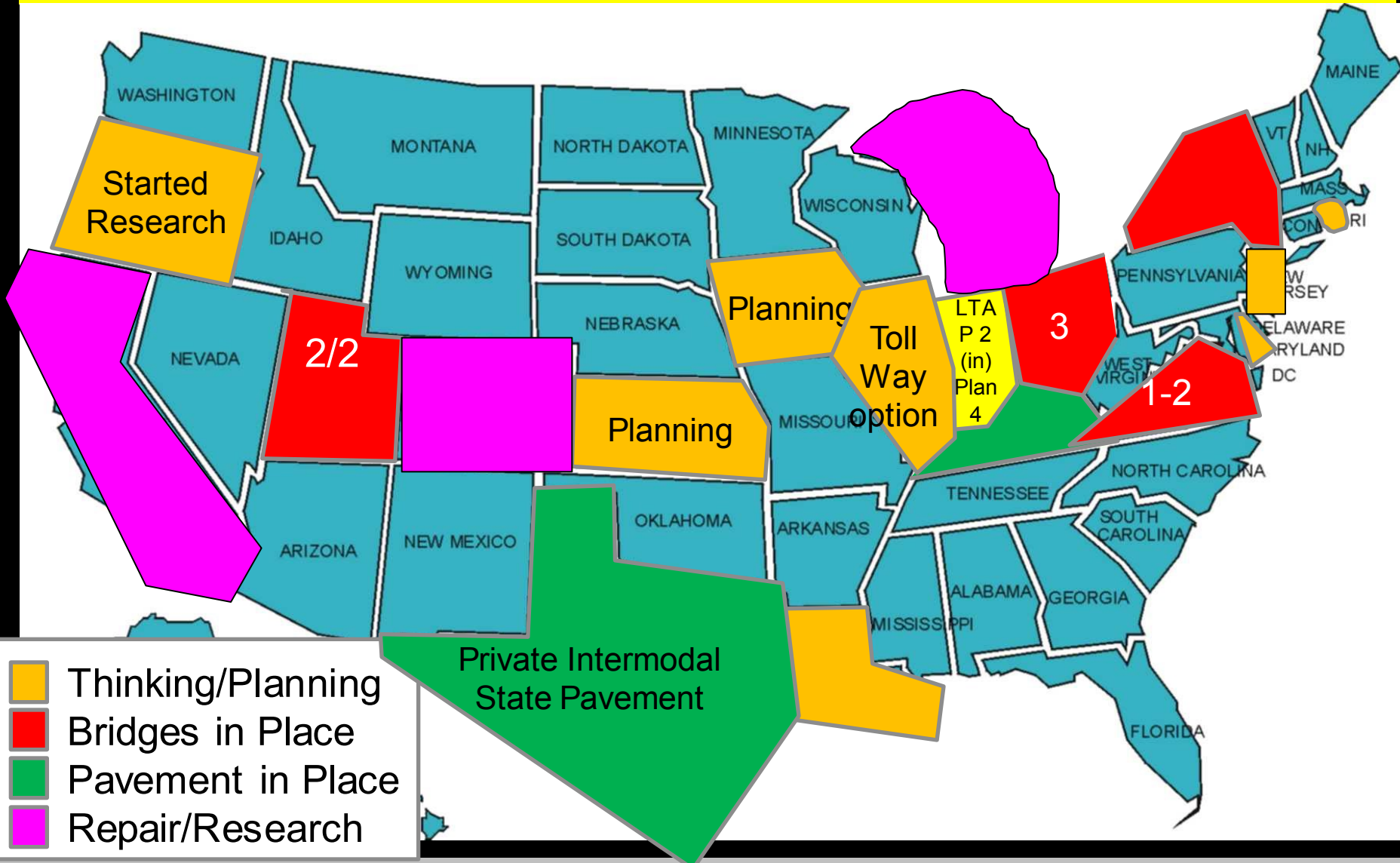


- Reduced connectivity of interfacial transition zones





# Internal Curing (Best Estimate)





# Indiana Field Trials - Conventional and IC mixtures in Sister Bridges



- Implemented in Monroe County Bridges in 2010
- Bridges cast using conventional ready mix concrete and conventional procedures
- Shows that this is a 'very off the shelf technology' – replace some FA with FLWA



# Monroe County IN (DiBella et al. 2012)

- Simple Change in Mixture Proportions

	Cement Content	W/C	Fine Aggregate	Fine LWA	Coarse Aggregate	Mixture Water	Water in LWA	WR	AE
	(kg/m <sup>3</sup> )		(kg/m <sup>3</sup> )	(kg/m <sup>3</sup> )	(kg/m <sup>3</sup> )	(kg/m <sup>3</sup> )	(kg/m <sup>3</sup> )	(%) <sup>A</sup>	(%) <sup>A</sup>
Plain Concrete	390	0.39	726	-	1046	152	-	0.22	0.08
Internally Cured Concrete	390	0.39	313	270	1046	152	25	0.22	0.08

- IN - Plain Slabs Cracked; IC did not crack

- IC has lower transport properties

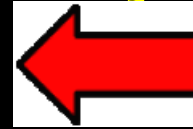
Time [days]	Charge Passed [Coulombs]			
	Plain Concrete	Standard Deviation	Internally Cured Concrete	Standard Deviation
28	4252	116	3822	159
56	2863	560	2458	55
91	3174	450	2065	113



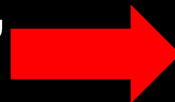
# Field comparison (Monroe Co In)



Plain bridge deck  
(Monroe Co.)  
1 year after casting.

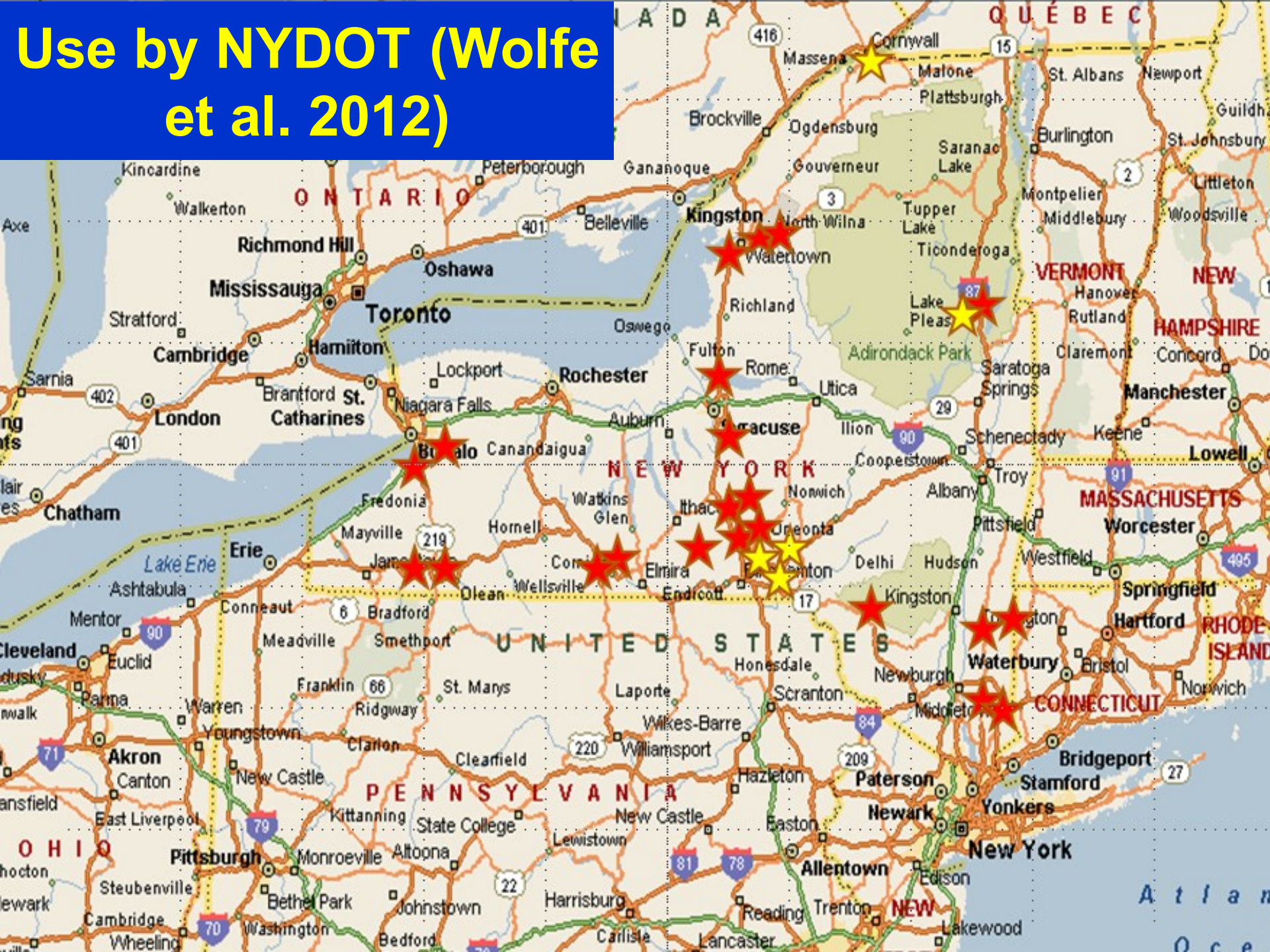


Internally cured bridge  
deck (Monroe Co.)  
“Crack Free 18+ mos  
after Casting”





Use by NYDOT (Wolfe  
et al. 2012)





# Tonawanda NY







# Comments from the Field

- Lanny Kipp, C&C Ready-Mix, NY
- Didn't see any issues batching, pumping, or finishing
- Soaking the stockpile for several days, mixing and blending is important for uniform moisture
- Remove the water source the day before to allow free water to drain
- Once the LWA is in the bins, loss to evaporation would be minimal
- LWA weighed first to insure proper amt
- Loading slowed slightly due to the addition of a 4<sup>th</sup> aggregate



**C & C Ready-Mix Corporation**  
Quality Building Products Since 1951



# Comments from the Field

- Jeff Jones, Buffalo Redi Mix, NY
- I saw no problems from the producer's, pumper's and even the contractor's perspective
- Air was not an issue pumping
- LWA worked fine in the plant. We ran 200 to 300 yards at a time.
- No issues with it hanging up
- I've worked with other low cement mixes and think that this is by far the best option
- I wouldn't hesitate to do it again
- Manual moisture, cooked 20 mins



BUFFALO CRUSHED STONE, INC.



# Tonwanda (190/I290) NY Results

- Similar RCPT results (DiBella et al 2011)  
\*note cond. LWA in test

	NY Lisle		NY Tonawanda	
Time [day]	Charge passing [Coulombs]		Charge passing [Coulombs]	
	Plain Concrete	IC Concrete	Plain Concrete	IC Concrete
28	535	423	572	570
56	373	406	342	313
91	357	392	308	301

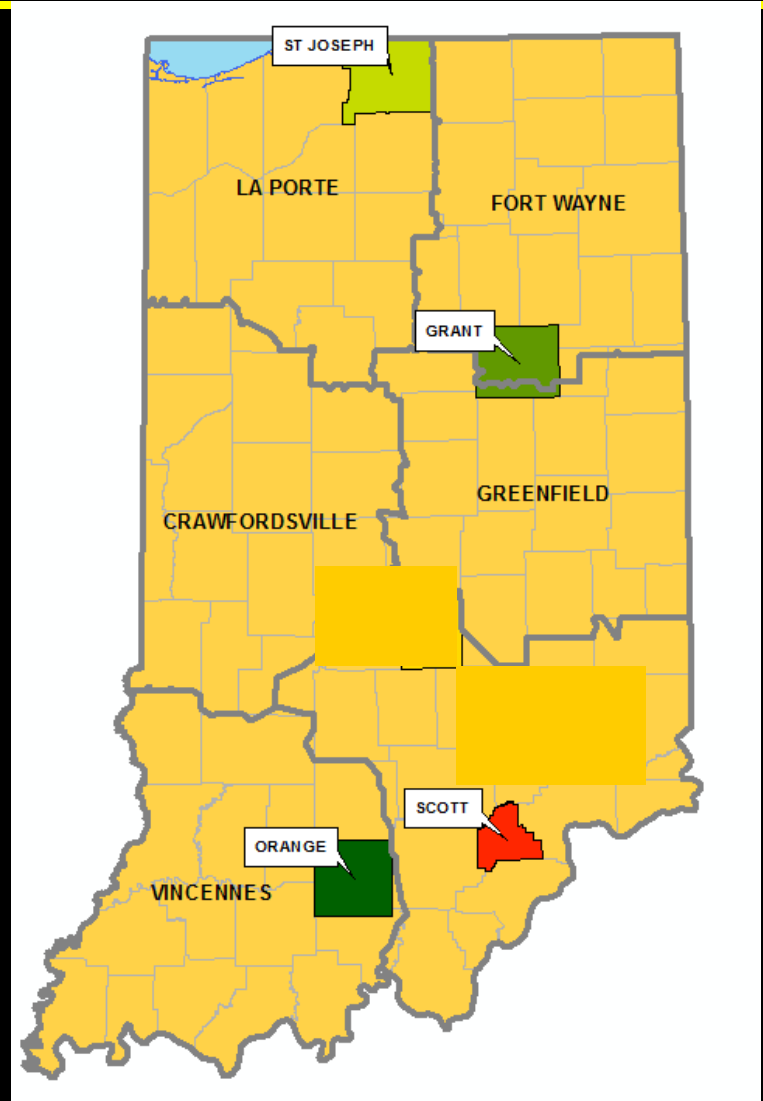
- Similar fresh properties
- Similar Strength (Wolfe et al 2012)

	Class HP	Class HP-IC
Comp. Str. 7 day	3,040 psi	3,500 psi
Comp. Str. 28 day	4,677 psi	4,683 psi
Comp. Str. 56 day	5,343 psi	5,417 psi
Concrete Density	140.2 pcf	135.2 pcf
Air Content	5.5 %	6.0 %
Slump	5.0"	4.5"



# ICHPC Coming to a County Near You for Full Scale Experiments (Carboneau)

- B-30498 SR 933, St. Joseph County, LaPorte District, October 2012
- B-33379 I-69, Grant County, Fort Wayne District, January 2013
- B-34199 US 150, Orange County, Vincennes District, February 2013
- B-35326 US 31, Scott Co, Seymour District, March 6<sup>th</sup>, 2013





# Summary

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- People want HSC due to low transport
- HSC can lead to increased cracking
- To reduce the potential for shrinkage cracking we can reduce paste volume, use a surface tension modifier (SRA) or use internal curing
- Internal curing – uses prewetted LWA
- Internal curing use is starting to increase
- There are 4 experimental projects occurring across Indiana (other states as well)